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ICC-ES Evaluation Report ESR-3449

DIVISION: 06 00 00—WOOD, PLASTICS AND

COMPOSITES

Section: 06 05 23—Wood, Plastic, and Composite

Fastenings

REPORT HOLDER:

MITEK® INC.

ADDITIONAL LISTEE:

THE HOME DEPOT

EVALUATION SUBJECT:

 MiTek^{\otimes} Structural connectors: Caps and bases

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2021, 2018, 2015, 2012, 2009 and 2006 *International Building Code*[®] (IBC)
- 2021, 2018, 2015, 2012, 2009 and 2006 International Residential Code[®] (IRC)

For evaluation for compliance with codes adopted by the Los Angeles Department of Building and Safety (LADBS), see ESR-3449 LABC and LARC Supplement.

Property evaluated:

Structural

2.0 USES

The MiTek structural connectors described in this report (see Table 10 for complete listing) are used for connecting wood framing members in accordance with Section 2304.10.4 of the 2021 IBC, Section 2304.10.3 of the 2018 and 2015 IBC, and Section 2304.9.3 of the 2012, 2009 and 2006 IBC. The connectors may also be used in structures regulated under the IRC when an engineered design is submitted to, and approved by, the code official, in accordance with Section R301.1.3 of the 2018, 2015, 2012, 2009 and 2006 IRC.

Reissued October 2022

This report is subject to renewal October 2024.

3.0 DESCRIPTION

3.1 C Post Cap:

The C Post Cap is designed to be installed on top of nominal dimension or rough sawn lumber posts for the attachment of solid-sawn beams. The C Post Cap is cold-formed from No. 18 gage steel and is prepunched for 16d common nails. See Table 1 and Figure 1 for product dimensions, fastener schedule, allowable loads, and a typical installation detail.

3.2 D Post Anchor:

The D Post Anchor is designed to secure nominal dimensioned or rough sawn posts to wood surfaces. The D Post Anchor is cold-formed from No. 18 gage steel and is prepunched for 16d common nails. See Table 2 and Figure 2 for product dimensions, fastener schedule, allowable loads, and a typical installation detail.

3.3 KCC and KECC Column Caps:

The KCC and KECC column caps are designed for beam-to-post connections. The KCC column cap provides connections for continuous beams, while the KECC column cap provides connections for end-of-beam configurations. The KCC and KECC column caps are fabricated from No. 7 gage or No. 3 gage hot-rolled steel plate. The legs of the column caps are attached to the U-shaped cap with factory-welded fillet welds. KCC and KECC column caps are attached to the post and beams utilizing either ⁵/₈- or ³/₄-inch-diameter (15.9 or 19.1 mm) bolts. See Table 3 and Figure 3 for product dimensions, fastener schedules, allowable loads, and a typical installation detail.

3.4 KCCQ and KECCQ Column Caps:

The KCCQ and KECCQ Column Caps are designed to connect wood beams to column posts utilizing MiTek Pro Series WS3 wood screws, which are supplied with the device. The column caps are fabricated from No. 7 gage or No. 3 gage hot-rolled steel, and are painted subsequent to fabrication. The connectors consist of a U-shaped plate, factory-welded to two vertical straps with $^3/_{16}$ -inch (4.8 mm) fillet welds. See Table 4 and Figure 4 for product dimensions, fastener schedule, allowable loads, and typical installation details.



3.5 PA Post Anchor:

The PA post anchor is designed to secure wood posts to concrete foundations. The anchor base is cold-formed from either No. 14 gage or No. 18 gage steel, and the stand-off plate is cold-formed from either No. 10 gage, No. 12 gage or No. 16 gage steel. The PA post anchor is prepunched for 16d common nails into the post, and must be used with either a $^{1}/_{2^{-}}$ or $^{5}/_{8^{-}}$ inch-diameter (12.7 or 15.9 mm) anchor bolt, as indicated in Table 5. See Table 5 and Figure 5 for product dimensions, fastener schedules, allowable loads, and typical installation details.

3.6 PAU Post Anchor:

The PAU Post Anchor is designed to secure wood posts to concrete or masonry members. The PAU Post Anchor is composed of three components: an anchor base, a stand-off plate, and a washer. The anchor base is cold-formed from No. 10 gage or No. 12 gage steel. The stand-off plate is cold-formed from either No. 12 gage or No. 16 gage steel. The washer is cut from No. 3 gage or No. 10 gage steel. The PAU Post Anchor is fastened to the post with either 16d common nails or 1/2-inch-diameter (12.7 mm) bolts. The PAU Post Anchor is fastened to the concrete or masonry foundation utilizing 5/8-inch-diameter (15.9 mm) anchor bolts, expansion anchors or threaded rod, which must be designed separately. See Table 6 and Figure 6 for product dimensions, required fastener schedule, allowable loads, and a typical installation detail.

3.7 PB, PBES and PBS Post Caps:

The PB, PBES and PBS Post Caps are designed to provide a post-to-beam connection of nominally dimensioned or rough sawn lumber. The PBES and PBS are required to be used in pairs (one on each side of the connection) at each connection location. The PB, PBES and PBS Post Caps are cold-formed from No. 18 gage steel and are prepunched for 16d common nails. See Table 7 and Figure 7 for product dimensions, fastener schedule, allowable loads, and typical installation details.

3.8 PCM and EPCM Post Caps:

The PCM and EPCM Post Caps are designed to provide a positive connection for post-to-beam applications. The post caps are cold-formed from either No. 12 gage or No. 16 gage steel, and are prepunched for 16d common nails. The PCM is configured for a beam that is continuous over the post, and the EPCM is configured for applications at the end of a beam. See Table 8 and Figure 8 for product dimensions, fastener schedule, allowable loads, and a typical installation detail.

3.9 PBC Post Beam Corner Connector:

The PBC Post Beam Corner Connector is a one-piece connector designed to secure two mitered beams on a corner post while providing uplift capacity. The connector is cold-formed from No. 18 gage steel, and is prepunched for 16d common nails. See Table 9 and Figure 9 for product dimensions, fastener schedule, allowable loads, and a typical installation detail.

3.10 Materials:

3.10.1 Steel: The specific types of steel and corrosion protection for each product are described in Table 10 of this report. Minimum steel base-steel thicknesses for the different gages are shown in the following table:

GAGE NO.	MINIMUM BASE-STEEL THICKNESS (inch)
18	0.044
16	0.055
14	0.070
12	0.099
10	0.129
7	0.171
3	0.240

For **SI**: 1 inch = 25.4 mm.

3.10.2 Wood: Wood members must be sawn lumber or structural glued laminated timber with a minimum specific gravity of 0.50, or approved structural engineered lumber (structural composite lumber, alternative strand lumber, or prefabricated wood I-joists) with a minimum equivalent specific gravity of 0.50, unless otherwise noted in the applicable table within this report. Wood members must have a moisture content not exceeding 19 percent (16 percent for structural engineered lumber), except as noted in Section 4.1. For connectors installed with nails, the thickness of each wood member must be sufficient such that the specified fasteners do not protrude through the opposite side of the member, unless otherwise permitted in the applicable table within this report. Wood members that are structural engineered lumber must be recognized in, and used in accordance with, a current evaluation report. Refer to Section 3.10.4 for issues related to treated wood.

3.10.3 Fasteners: Required fastener types and sizes for use with the MiTek connectors described in this report are specified in this section and Tables 1 through 9.

3.10.3.1 MiTek Pro Series Wood Screws: The wood screws used with the KCCQ and KECCQ connectors must be MiTek Pro Series WS3 wood screws, as described in <u>ESR-2761</u>. The appropriate size of MiTek Pro Series WS wood screws must be used, as indicated in the applicable tables of this report.

3.10.3.2 Bolts: At a minimum, bolts must comply with ASTM A307 and must have a minimum bending yield strength of 45,000 lbf/in.² (310 MPa). Bolt diameters must be as specified in the applicable tables of this report.

3.10.3.3 Nails: Nails used for connectors described in this report must be bright or hot-dipped galvanized carbon steel nails complying with material requirements, physical properties, tolerances, workmanship, protective coating and finishes, and packaging and package marking requirements specified in ASTM F1667; and must have lengths, diameters and bending yield strengths as shown in the following table:

FASTENER DESIGNATION	FASTENER LENGTH (inches)	SHANK DIAMETER (inch)	MINIMUM REQUIRED F _{yb} (lbf/in²)
16d common	3.5	0.162	90,000
1/2" dia. bolt	Varies	0.500	45000
⁵ / ₈ " dia. bolt	Varies	0.625	45000
3/4" dia. bolt	Varies	0.750	45000

For **SI:** 1 inch = 25.4 mm, 1 psi = 6.895 kPa.

Alternatively, nails of other materials or finishes may be used when they are recognized in an ICC-ES evaluation report as having bending yield strength and withdrawal capacity equal to or better than those of a bright carbon steel of the same nominal diameter.

3.10.4 Use in Treated Wood: Connectors fasteners used in contact with preservative-treated or fire-retardant-treated wood must comply with Section 2304.10.6 of the 2021 IBC and Section 2304.10.5 of the 2018 and 2015 IBC (Section 2304.9.5 of the 2012, 2009 and 2006 IBC) and Section R317.3 IRC (Section R319.3 of the 2006 IRC). The lumber treater or the report holder (MiTek), or both, should be contacted for recommendations on the appropriate level of corrosion resistance to specify for the connectors and fasteners as well as the connection capacities of the fasteners used with the specific proprietary preservative-treated or fire-retardant-treated lumber.

3.10.5 Concrete and Masonry Construction: Materials and quality of concrete and masonry construction must comply with the applicable provisions of Chapter 19 and 21 of the IBC. The compressive strength of the concrete and masonry construction must be in accordance with the approved design and applicable provisions of the building code.

4.0 DESIGN AND INSTALLATION

4.1 Design:

The allowable load capacities in Tables 1 through 9 are based on allowable stress design. The use of the allowable load values for the products listed in Table 11 of this report must comply with all applicable requirements and conditions specified in this report. Tabulated allowable loads are for normal load duration and/or short load duration, based on load duration factors, C_D, in accordance with Section 11.3.2 of the 2018 and 2015 ANSI/AWC National Design Specification® for Wood Construction (NDS) (Section 10.3.2) of the 2012 NDS), as indicated in Tables 1 through 9 of this report. No further increases are permitted for load durations other than those specified. Tabulated allowable loads are for connections in wood seasoned to a maximum moisture content of 19 percent (16 percent for engineered wood products) or less, used under continuously dry conditions and where sustained temperatures are limited to 100°F (37.8°C) or less. When connectors are installed in wood having a moisture content greater than 19 percent (16 percent for engineered wood products), or where the inservice moisture content is expected to exceed this value, the applicable wet service factor, C_M, must be applied. Unless otherwise noted in the tables of this report, the applicable wet service factor, C_M, is as specified in the NDS for lateral loading of dowel-type fasteners. When connectors are installed in wood that will experience sustained exposure to temperatures exceeding 100°F (37.8°C), the allowable loads in this evaluation report must be adjusted by the temperature factor, Ct, specified in Section 11.3.4 of the 2018 and 2015 NDS (Section 10.3.4 of the 2012 NDS). The group action factor, Cg, has been accounted for, in accordance with Section 11.3.6 of the 2018 and 2015 NDS (Section 10.3.6 of the 2012 NDS), in the tabulated allowable loads, where applicable. For connectors installed with bolts, minimum edge distances and end distances within the wood members must be met, such that the geometry factor, C_{Δ} , is 1.0, in accordance with Section 12.5.1 of the 2018 and 2015 NDS (Section 11.5.1 of the 2012 NDS), unless otherwise noted in this report. Connected wood members must be checked for load-carrying capacity at the connection in accordance with Section 11.1.2 of the 2018 and 2015 NDS (Section 10.1.2 of the 2012 NDS).

4.2 Installation:

Installation of the connectors must be in accordance with this evaluation report and the manufacturer's published installation instructions. Mechanical fasteners must be installed in wood members in accordance with Section 12.1 of the 2018 and 2015 NDS (Section 11.1 of the 2012 NDS).

4.3 Special Inspection:

4.3.1 Main Wind-force-resisting Systems under the IBC: Periodic special inspection must be conducted for components within the main wind-force-resisting system, where required in accordance with Sections 1704.2 and 1705.12 of the 2021 IBC; Sections 1704.2 and 1705.11 of the 2018 and 2015 IBC, 1705.10 of the 2012 IBC, Sections 1704 and 1706 of the 2009 IBC, and Section 1704 of the 2006 IBC.

4.3.2 Seismic-force-resisting Systems under the IBC: Periodic special inspection must be conducted for components within the seismic-force-resisting system, where required in accordance with Sections 1704.2 and 1705.13 of the 2021 IBC; Section 1704.2 and 1705.12 of the 2018 and 2015 IBC, 1705.11 of the 2012 IBC, and Sections 1704 and 1707 of the 2009 and 2006 IBC.

4.3.3 Installations under the IRC: Special inspections are normally not required for connectors used in structures regulated under the IRC. However, for components and systems requiring an engineered design in accordance with IRC Section R301, periodic special inspection requirements and exemptions must be in accordance with Sections 4.3.1 and 4.3.2 of this report.

5.0 CONDITIONS OF USE

The MiTek® Structural Connectors described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1 The connectors must be manufactured, identified and installed in accordance with this report and the manufacturer's published installation instructions. A copy of the manufacturer's published installation instructions must be available at the jobsite at all times during installation. In the event of a conflict between this report and the manufacturer's published installation instructions, this report governs.
- 5.2 Calculations showing compliance with this report must be submitted to the code official. The calculations must be prepared by a registered design professional where required by the statutes of the jurisdiction in which the project is to be constructed.
- 5.3 Connected wood members and fasteners must comply with Sections 3.10.2 and 3.10.3, respectively.
- 5.4 Adjustment factors, noted in Section 4.1 of this report and the applicable codes, must be considered where applicable.
- 5.5 Use of connectors and fasteners with preservativetreated or fire-retardant-treated lumber must be in accordance with Section 3.10.4.
- **5.6** The design of the anchorage to, and bearing upon, concrete or masonry construction, inclusive of cast-inplace and post-installed anchors, used to attach the connectors described in this report to concrete or masonry construction, is outside the scope of this report.
- Connectors with factory welds are identified in Table 10 as being manufactured at the designated facilities under a quality control program with inspections by ICC-ES.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Joist Hangers and Similar Devices (AC13), approved October 2018 (editorially revised December 2020).

7.0 IDENTIFICATION

- 7.1 Each connector described in this report is identified by the product model (stock) number, the number of the ICC-ES index evaluation report for MiTek (ESR-2685), and by one or more of the following designations: MiTek, USP or USP Structural Connectors.
- **7.2** The report holder's contact information is the following:

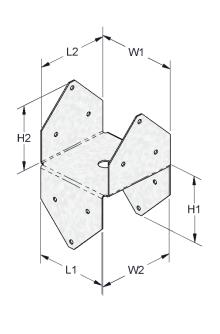
MITEK® INC. 16023 SWINGLEY RIDGE ROAD CHESTERFIELD, MISSOURI 63017 800-328-5934 www.mitek-us.com uspcustomerservice@mii.com 7.3 The additional Listee's contact information is the following:

THE HOME DEPOT
2455 PACES FERRY ROAD SE
ATLANTA, GEORGIA 30339
(678) 216-8204
www.homedepot.com

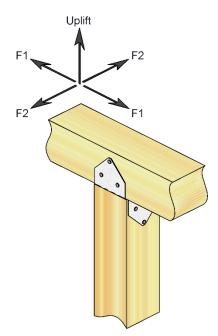
TABLE 1—C POST CAP ALLOWABLE LOADS^{1,2,3}

			DIM	ENSIC	NS (ir	nches)			FASTENE	R SCHEDULE	ALLOWABLE LOADS (lbs)		
STOCK NO.	STEEL GAGE	W1	W2	L1	L2	H1	H2	Ó	Qty.	Nail Type	Uplift	Lateral (F1 & F2)	
								Post	Beam		C _D = 1.6	C _D = 1.6	
C44	18	3 ⁹ / ₁₆	39/16	31/4	31/4	27/8	27/8	6	6	16d Common	925	1,105	
C44R	18	4	4	31/4	31/4	2 ⁵ / ₈	2 ⁵ / ₈	8	8	16d Common	925	1,105	
C46	18	3 ⁹ / ₁₆	5 ¹ / ₂	33/8	5 ¹ / ₄	2 ⁹ / ₁₆	2 ⁵ / ₈	6	10	16d Common	925	1,105	
C46R	18	4	6	31/4	5 ¹ / ₄	23/4	23/4	8	10	16d Common	925	1,105	
C66	18	5 ¹ / ₂	5 ¹ / ₂	5 ¹ / ₂	5 ¹ / ₄	27/8	2 ⁷ / ₈	12	12	16d Common	1,195	2,100	
C66R	18	6	6	51/4	5 ¹ / ₄	2 ¹³ / ₁₆	2 ¹³ / ₁₆	10	10	16d Common	955	2,210	
C88	18	71/2	71/2	73/8	73/8	5	5	16	16	16d Common	1,195	2,260	
C88R	18	8	8	73/8	73/8	5	5	16	16	16d Common	1,195	2,260	

³Allowable loads shown are for installations in wood members complying with Section 3.10.2.







Typical C Post Cap Installation

¹Allowable loads have been adjusted for a load duration factor, C_D, of 1.6, corresponding to a ten-minute load duration (i.e., wind or earthquake loading), in accordance with the NDS. The allowable loads do not apply to loads of other durations. See Sections 4.1 and 4.2 for design and installation requirements.

²See Section 3.10.3 for required fastener dimensions and mechanical properties.

TABLE 2—D POST ANCHOR ALLOWABLE LOADS 1,2,3,4

		DIMENS	SIONS (i	nches)		FASTENE	R SCHEDULE	ALLOW	ABLE LOA	DS (lbs)
STOCK NO.	STEEL GA.	w	н	L	(Qty.	Noil Type	F1	F2	Uplift
110.	Č	VV	п	L	Post	Beam	Nail Type	C _D = 1.6	C _D = 1.6	C _D = 1.6
D44-TZ	18	3 ⁹ / ₁₆	21/2	33/8	8	4	16d Common	885	885	700
D44R	18	4	3	33/4	8	4	16d Common	885	885	700
D46	18	39/16	3	5 ³ / ₈	10	5	16d Common	995	1,095	700
D46R-TZ	18	4	3	53/8	10	5	16d Common	995	1,095	700
D66	18	5 ¹ / ₂	3	5 ³ / ₈	10	5	16d Common	995	1,095	700
D66R	18	6	3	5 ³ / ₈	10	5	16d Common	995	1,095	700
D88	18	71/2	3	7 ³ / ₈	12	5	16d Common	995	1,095	700
D88R	18	8	3	73/8	12	5	16d Common	995	1,095	700

⁵When connectors are installed in wood having a moisture content greater than 19 percent (16 percent for engineered wood products), or where the moisture content is expected to exceed this value at any time while in service, the allowable uplift load must be adjusted by the applicable wet service factor, C_M, as specified for withdrawal loads of nails and spikes in the NDS. Wet service factors, C_M, applicable to the F1 and F2 load directions are as specified for lateral loads of dowel-type fasteners in the NDS.

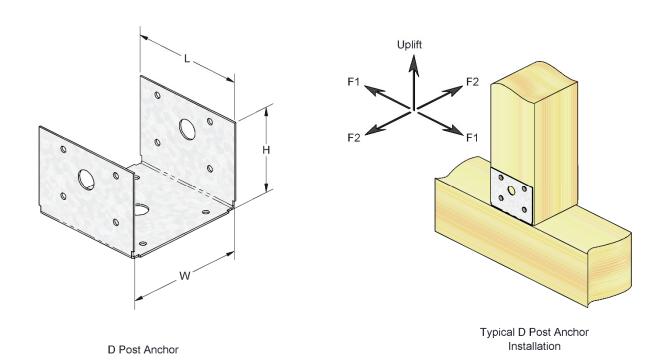


FIGURE 2—D POST ANCHOR

¹Allowable loads have been adjusted for a load duration factor, C_D, of 1.6, corresponding to a ten-minute load duration (i.e., wind or earthquake loading), in accordance with the NDS. The allowable loads do not apply to loads of other durations. See Sections 4.1 and 4.2 for design and installation requirements.

²See Section 3.10.3 for required fastener dimensions and mechanical properties.

³ Allowable loads shown are for installations in wood members complying with Section 3.10.2.

⁴The supporting beam must have a minimum dimension of 3.5 inches (89 mm) in the direction parallel to the nail axis.

TABLE 3—KCC AND KECC COLUMN CAP ALLOWABLE LOADS^{1,2,3}

	WOOD M	EMBER				IMEN	SIONS	3 (in.)			ВС	DLT SC	CHEDU	LE		ALL	OWABLE	E LOADS (lbs)	
OTOOK NO	Beam	Post	픾띪								Ве	am		_	-4	Beari	ng ^{5,6}	Up	lift ^{7,8}
STOCK NO.	Width	Width	STEEL GAGE	W1	W2 ⁴	H1	H2		L	K	CC	KE	CC	P	ost	C _D =	1.0	C _D	= 1.6
	(in)	(in)						KCC	KECC	Qty	Dia.	Qty	Dia.	Qty	Dia.	KCC	KECC	KCC	KECC
KCC325-4	31/8	31/2	7	31/4	35/8	6 ¹ / ₂	81/2	11	71/2	4	5/8	2	5/8	2	5/8	21,485	14,650	3,505	1,750
KCC325-6	31/8	5 ¹ / ₂	7	31/4	51/2	61/2	81/2	11	71/2	4	5/8	2	5/8	2	5/8	21,485	14,650	3,505	1,750
KCC44	31/2	31/2	7	35/8	35/8	4	81/2	7	5 ¹ / ₂	2	⁵ / ₈	1	5/8	2	5/8	15,315	12,030	3,920	1,960
KCC45	31/2	5 ¹ / ₄	7	35/8	5 ¹ / ₂	6 ¹ / ₂	81/2	11	71/2	4	⁵ / ₈	2	5/8	2	5/8	24,065	16,405	3,920	1960
KCC46	31/2	5 ¹ / ₂	7	35/8	5 ¹ / ₂	61/2	81/2	11	81/2	4	5/8	2	5/8	2	5/8	24,065	18,595	3,920	1,960
KCC47	31/2	7	7	35/8	71/8	61/2	81/2	11	91/2	4	5/8	2	5/8	2	5/8	24,065	20,780	3,920	1,960
KCC48	31/2	71/2	7	35/8	71/2	61/2	81/2	11	91/2	4	5/8	2	5/8	2	5/8	24,065	20,780	3,920	1,960
KCC525-4	5 ¹ / ₈	31/2	3	5 ¹ / ₄	35/8	8	81/2	13	91/2	4	3/4	2	3/4	2	3/4	41,640	30,430	8,155	6,050
KCC525-6	5 ¹ / ₈	5 ¹ / ₂	3	5 ¹ / ₄	5 ¹ / ₂	8	81/2	13	91/2	4	3/4	2	3/4	2	3/4	41,640	30,430	8,155	6,050
KCC525-8	5 ¹ / ₈	71/2	3	5 ¹ / ₄	71/2	8	81/2	13	91/2	4	3/4	2	3/4	2	3/4	41,640	30,430	8,155	6,050
KCC57	5 ¹ / ₄	7	7	5 ³ / ₈	71/8	6 ¹ / ₂	81/2	11	91/2	4	⁵ / ₈	2	5/8	2	5/8	36,095	31,170	4,210	2,105
KCC64	51/2	31/2	7	5 ¹ / ₂	35/8	61/2	81/2	11	71/2	4	5/8	2	5/8	2	5/8	37,815	25,780	4,210	2,105
KCC66	5 ¹ / ₂	5 ¹ / ₂	7	5 ¹ / ₂	5 ¹ / ₂	6 ¹ / ₂	81/2	11	71/2	4	5/8	2	5/8	2	5/8	37,815	25,780	4,210	2,105
KCC68	5 ¹ / ₂	71/2	7	5 ¹ / ₂	71/2	6 ¹ / ₂	81/2	11	91/2	4	⁵ / ₈	2	5/8	2	5/8	37,815	32,655	4,210	2,105
KCC74	63/4	31/2	3	6 ⁷ / ₈	35/8	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	54,845	44,295	8,155	6,050
KCC75X	7	5 ¹ / ₄	3	71/8	5 ¹ / ₂	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	56,875	45,940	8,155	6,050
KCC76	63/4	5 ¹ / ₂	3	6 ⁷ / ₈	5 ¹ / ₂	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	54,845	44,295	8,155	6,050
KCC77	63/4	63/4	3	6 ⁷ / ₈	6 ⁷ / ₈	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	54,845	44,295	8,155	6,050
KCC77X	7	7	3	71/8	71/8	8	8 ¹ / ₂	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	56,875	45,940	8,155	6,050
KCC78	63/4	71/2	3	6 ⁷ / ₈	71/2	8	8 ¹ / ₂	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	54,845	44,295	8,155	6,050
KCC84	71/2	31/2	3	71/2	35/8	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	60,940	49,220	8,155	6,050
KCC86	71/2	5 ¹ / ₂	3	71/2	5 ¹ / ₂	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	60,940	49,220	8,155	6,050
KCC88	71/2	71/2	3	71/2	71/2	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	60,940	49,220	8,155	6,050
KCC94	83/4	31/2	3	87/8	35/8	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	71,095	57,420	8,155	6,050
KCC96	83/4	5 ¹ / ₂	3	8 ⁷ / ₈	5 ¹ / ₂	8	8 ¹ / ₂	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	71,095	57,420	8,155	6,050
KCC98	83/4	71/2	3	8 ⁷ / ₈	71/2	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	71,095	57,420	8,155	6,050
KCC106	91/2	51/2	3	95/8	5 ¹ / ₂	8	81/2	13	10 ¹ / ₂	4	3/4	2	3/4	2	3/4	77,190	62,345	8,155	6,050

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.895 kPa.

¹Allowable loads have been adjusted for load duration factors, C_D , as shown, in accordance with the NDS. The allowable loads do not apply to loads of other durations, and are not permitted to be adjusted for other load durations. See Sections 4.1 and 4.2 for additional design and installation requirements.

²See Section 3.10.3 for required fastener dimensions and mechanical properties.

³Allowable loads shown are for installations in wood members complying with Section 3.10.2. Wood members must also have a minimum reference compression perpendicular to grain design value, F_{c-perp}, of 625 psi (4.31 MPa).

⁴Values indicate the required dimension of the post in the direction parallel to the long axis of the beam.

⁵Allowable bearing loads apply to conditions in which the underside of the beam and top of the post are in contact with the steel bearing surfaces of the column cap, and the beam is continuous over the length, *L*, of the column cap.

⁶Allowable bearing loads must be further reduced to account for stress limitations in the column (including axial loading and moment due to eccentricity), where such limitations govern.

⁷Allowable uplift loads for the KCC column caps do not apply to spliced beam configurations.

 $^{^8}$ Allowable uplift loads have been adjusted to account for geometry factors, C_4 , of less than 1.0, due to end distance constraints for the bolts within the post.

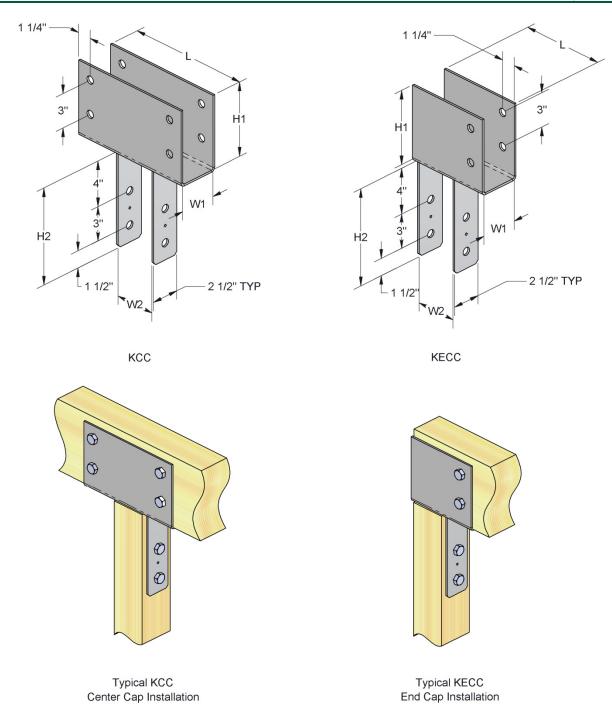


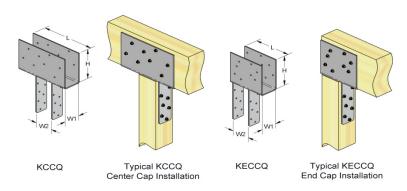
FIGURE 3—KCC AND KECC COLUMN CAPS

TABLE 4—KCCQ AND KECCQ COLUMN CAP ALLOWABLE LOADS^{1,2,3}

	WOOD M	EMBERS		DII	MENSI	ONS (i	inches)		NO. OF	WS3		ALLOWAB	LE LOADS (I	bs.)
07001/110								L	SCRE	EWS	Dowr	nload ^{5,6}	Up	lift ⁷
STOCK NO.	Beam Width	Post Size⁴	Steel Gauge	W1	W2 ⁴	н	14000	КЕООО	D	D 4	C _D	= 1.0	C _D =	= 1.6
	widti	Size	Gauge				KCCQ	KECCQ	Beam	Post	KCCQ	KECCQ	KCCQ	KECCQ
KCCQ325-4	3 ¹ / ₈	31/2	7	31/4	35/8	6 ¹ / ₂	11	71/2	16	14	21,485	14,650	7,065	6,860
KCCQ325-6	31/8	5 ¹ / ₂	7	31/4	5 ¹ / ₂	6 ¹ / ₂	11	71/2	16	14	21,485	14,650	7,065	6,860
KCCQ44	31/2	31/2	7	35/8	35/8	6 ¹ / ₂	11	81/2	16	14	24,065	16,965	7,065	6,860
KCCQ45	31/2	5 ¹ / ₂	7	35/8	5 ¹ / ₂	6 ¹ / ₂	11	71/2	16	14	24,065	16,405	7,065	6,860
KCCQ46	31/2	5 ¹ / ₂	7	35/8	5 ¹ / ₂	6 ¹ / ₂	11	8 ¹ / ₂	16	14	24,065	18,595	7,065	6,860
KCCQ47	31/2	7	7	35/8	71/8	6 ¹ / ₂	11	91/2	16	14	24,065	20,780	7,065	6,860
KCCQ47X	31/2	7	3	35/8	7 ¹ / ₈	8	13	91/2	16	14	28,440	20,780	7,065	6,860
KCCQ48	31/2	71/2	7	35/8	71/2	6 ¹ / ₂	11	8 ¹ / ₂	16	14	24,065	18,595	7,065	6,860
KCCQ525-4	5 ¹ / ₈	31/2	3	5 ¹ / ₄	35/8	8	13	91/2	16	14	41,640	22,330	7,065	6,860
KCCQ525-6	5 ¹ / ₈	5 ¹ / ₂	3	5 ¹ / ₄	5 ¹ / ₂	8	13	9 ¹ / ₂	16	14	41,640	27,300	7,065	6,860
KCCQ525-8	5 ¹ / ₈	71/2	3	5 ¹ / ₄	71/2	8	13	91/2	16	14	41,640	30,430	7,065	6,860
KCCQ57	5 ¹ / ₄	7	7	5 ³ / ₈	71/8	61/2	11	91/2	16	14	36,095	31,170	7,065	6,860
KCCQ64	5 ¹ / ₂	31/2	7	5 ¹ / ₂	35/8	6 ¹ / ₂	11	8 ¹ / ₂	16	14	37,815	23,535	7,065	6,860
KCCQ66	5 ¹ / ₂	5 ¹ / ₂	7	5 ¹ / ₂	5 ¹ / ₂	6 ¹ / ₂	11	8 ¹ / ₂	16	14	37,815	28,910	7,065	6,860
KCCQ67X	5 ¹ / ₂	7	7	5 ¹ / ₂	71/8	61/2	11	81/2	16	14	37,815	29,220	7,065	6,860
KCCQ68	5 ¹ / ₂	71/2	7	5 ¹ / ₂	71/2	6 ¹ / ₂	11	8 ¹ / ₂	16	14	37,815	29,220	7,065	6,860
KCCQ71-4	7	31/2	3	71/8	35/8	6 ¹ / ₂	11	8 ¹ / ₂	16	14	48,125	28,240	7,065	6,860
KCCQ71-6	7	5 ¹ / ₂	3	71/8	5 ¹ / ₂	6 ¹ / ₂	11	81/2	16	14	48,125	35,285	7,065	6,860
KCCQ71-71	7	7	3	71/8	7 ¹ / ₈	6 ¹ / ₂	11	8 ¹ / ₂	16	14	48,125	37,190	7,065	6,860
KCCQ71-8	7	71/2	3	71/8	71/2	6 ¹ / ₂	11	8 ¹ / ₂	16	14	48,125	37,190	7,065	6,860
KCCQ74	63/4	31/2	3	6 ⁷ / ₈	35/8	6 ¹ / ₂	11	81/2	16	14	46,405	27,465	7,065	6,860
KCCQ76	63/4	6 ¹ / ₂	3	6 ⁷ / ₈	5 ¹ / ₂	6 ¹ / ₂	11	8 ¹ / ₂	16	14	46,405	35,860	7,065	6,860
KCCQ77	63/4	63/4	3	6 ⁷ / ₈	6 ⁷ / ₈	6 ¹ / ₂	11	8 ¹ / ₂	16	14	46,405	35,860	7,065	6,860
KCCQ78	63/4	71/2	3	6 ⁷ / ₈	71/2	61/2	11	81/2	16	14	46,405	35,860	7,065	6,860
KCCQ84	71/2	31/2	7	71/2	35/8	6 ¹ / ₂	11	8 ¹ / ₂	16	14	51,565	29,785	7,065	6,860
KCCQ86	71/2	5 ¹ / ₂	7	71/2	5 ¹ / ₂	6 ¹ / ₂	11	8 ¹ / ₂	16	14	51,565	37,390	7,065	6,860
KCCQ88	71/2	71/2	7	71/2	71/2	61/2	11	81/2	16	14	51,565	39,845	7,065	6,860
KCCQ94	83/4	31/2	7	8 ⁷ / ₈	35/8	61/2	11	8 ¹ / ₂	16	14	60,155	33,595	7,065	6,860
KCCQ96	83/4	5 ¹ / ₂	7	8 ⁷ / ₈	5 ¹ / ₂	6 ¹ / ₂	11	8 ¹ / ₂	16	14	60,155	42,630	7,065	6,860
KCCQ98	83/4	71/2	7	87/8	71/2	61/2	11	81/2	16	14	60,155	46,485	7,065	6,860
KCCQ106	9 ¹ / ₂	5 ¹ / ₂	7	91/2	5 ¹ / ₂	6 ¹ / ₂	11	8 ¹ / ₂	16	14	65,315	45,760	7,065	6,860

For **SI:** 1 inch = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.89 kPa.

⁷Allowable uplift loads for the KCCQ column caps do not apply to spliced beam configurations.



¹Allowable loads have been adjusted for load duration factors, C_D, as shown, in accordance with the NDS. The allowable loads do not apply to loads of other durations, and are not permitted to be adjusted for other load durations. See Sections 4.1 and 4.2 for additional design and installation requirements.

²See Section 3.10.3 for required fastener dimensions and mechanical properties.

³Allowable loads shown are for installations in wood members complying with Section 3.10.2 Wood members must also have a reference compression perpendicular to grain design value, F_{c-perp}, of 625 psi (4.31 MPa) or greater.

⁴Values indicate the required dimension of the post in the direction parallel to the long axis of the beam.

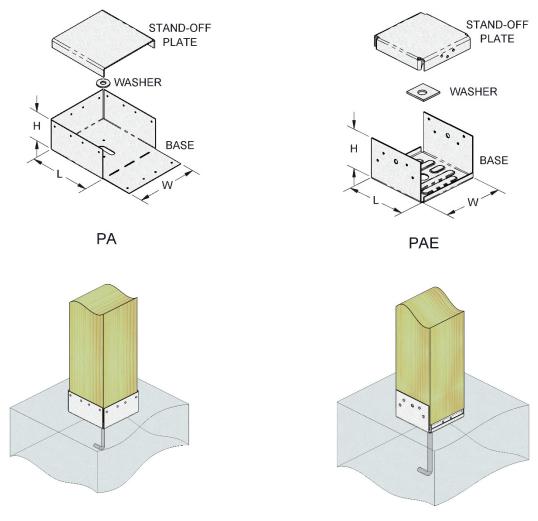
⁵Allowable downloads apply to conditions in which the underside of the beam and the top of the post are in contact with the steel bearing surfaces of the column cap, and the beam is continuous over the length, L, of the column cap.

⁶Allowable downloads must be further reduced to account for stress limitations in the column (including axial loading and moment due to eccentricity), where such limitations govern.

TABLE 5—PA POST ANCHOR ALLOWABLE LOADS 1,2,3,4,5

	STE	EL GAGE	DIMI	ENSIONS	(in)		FASTENE	R SCHE	DULE	ALLOWABLI	E LOADS (lbs)
STOCK NO.		Stand-off	w			Anch	or Bolt (in.)		Post	Bearing	Uplift
110.	Base	Plate	w		Н	Qty.	Туре	Qty.	Туре	C _D = 1.0	C _D = 1.6
PA44	18	12	39/16	31/2	21/8	1	1/2	8	16d Common	4,155	455
PA44R	18	12	41/16	4	31/2	1	1/2	12	16d Common	4,155	455
PA46	18	12	39/16	5 ¹ / ₂	31/2	1	1/2	14	16d Common	4,155	455
PA46R	18	10	41/16	6	31/2	1	1/2	14	16d Common	4,155	455
PA66	18	12	5 ⁹ / ₁₆	5 ¹ / ₂	31/2	1	1/2	16	16d Common	5,930	250
PA66R	18	12	6 ¹ / ₁₆	6 ¹ / ₁₆	31/4	1	1/2	16	16d Common	5,930	250
PA44E	18	16	39/16	31/2	31/2	1	1/2	6	16d Common	6,775	1,035
PA46E	18	12	39/16	5 ¹ / ₂	31/2	1	5/8	8	16d Common	6,775	1,035
PA66E	14	12	5 ¹ / ₂	5 ¹ / ₂	31/2	1	5/8	8	16d Common	16,005	1,130
PA66ER-TZ	14	12	6	5 ¹ / ₂	31/4	1	5/8	8	16d Common	16,005	1,130

⁵The PA Post Anchor has no moment or lateral capacity and should not be used in fixed post applications.



Typical PA Installation

Typical PAE Installation

¹Allowable loads have been adjusted for load duration factors, C_D, as shown, in accordance with the NDS. The allowable loads do not apply to loads of other durations, and are not permitted to be adjusted for other load durations. See Sections 4.1 and 4.2 for additional design and installation requirements.

²See Section 3.10.3 for required fastener dimensions and mechanical properties.

³Allowable loads shown are for installations in wood members complying with Section 3.10.2.

⁴Allowable loads shown do not apply to the anchorage to concrete or masonry. Anchorage to concrete or masonry must be designed by a registered design professional in accordance with Section 4.1 of this report.

TABLE 6—PAU POST ANCHOR^{1,2,3,4,5,6}

	,	STEEL GAG	GE	D	imensio	าร		FASTENEI	R SCHI	EDULE			ALLOW	WABLE LOADS (lbs)		
STOCK					(in.)			Nails	Во	lts	And	chor	Beering	Uplift		
NO.	Base	Stand- Off Plate	Washer	w	н			(in.)	(iı	n.)	Вс	lts	Bearing	Nails	Bolts	
		On Flate		VV	г	L	Qty	Qty Type (Size	Qty	Size	100%	C _D = 1.6	C _D = 1.6	
PAU44	12	16	10	3 ⁹ / ₁₆	5 ⁷ / ₁₆	3	12	16d Common	2	1/2	1	5/8	6,775	2,535	2,265	
PAU46	10	12	10	3 ⁹ / ₁₆	6	5	12	16d Common	2	1/2	1	⁵ / ₈	13,815	2,535	2,265	
PAU66	10	12	10	5 ¹ / ₂	6	5	12	16d Common	2	1/2	1	5/8	16,005	2,455	2,265	
PAU66R	10	12	10	6 ¹ / ₁₆	53/4	5	12	16d Common	2	1/2	1	⁵ / ₈	16,005	1,475	1,475	
PAU88	12	12	3	71/2	73/16	71/16	14	16d Common	-		2	5/8	24,900	3,315		
PAU88R	12	12	3	81/16	6 ¹⁵ / ₁₆	71/16	14	16d Common			2	5/8	24,900	3,315		
PAU1010	12	16	10	91/2	73/16	91/2	14	16d Common	2	⁵ / ₈	2	⁵ / ₈	27,095	1,495	1,495	
PAU1010R	12	16	10	10 ¹ / ₁₆	73/16	10	14	16d Common	2	⁵ / ₈	2	⁵ / ₈	27,095	1,495	1,495	
PAU1212	12	12	10	11 ¹ / ₂	6 ⁷ / ₈	11 ¹ / ₂	18 16d Common		2	⁵ / ₈	2	⁵ / ₈	64,015	1,180	1,180	
PAU1212R	12	12	10	12 ¹ / ₈	6 ⁷ / ₈	12 ¹ / ₈	18 16d Common		2	⁵ / ₈	2	⁵ / ₈	64,015	1,180	1,180	

⁶Allowable nail loads and bolt loads are not permitted to be added together.

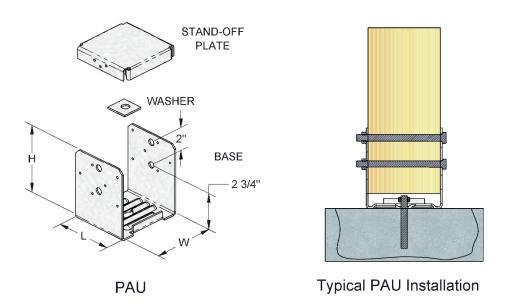


FIGURE 6—PAU POST ANCHOR

¹Allowable loads have been adjusted for load duration factors, C_D, as shown, in accordance with the NDS. The allowable loads do not apply to loads of other durations, and are not permitted to be adjusted for other load durations. See Sections 4.1 and 4.2 for additional design and installation requirements. ²See Section 3.10.3 or required fastener dimensions and mechanical properties.

³Allowable loads shown are for installations in wood members complying with Section 3.10.2. Values apply only to parallel-to-grain loading of wood members.

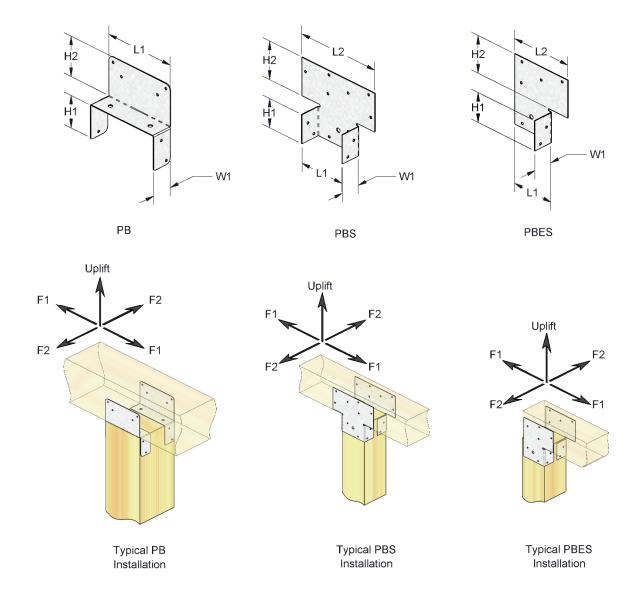
⁴Allowable loads shown do not apply to the anchorage to concrete or masonry. Anchorage to concrete or masonry must be designed by a registered design professional in accordance with Section 4.1 of this report.

⁵The PAU Post Anchor has no moment or lateral capacity and should not be used in fixed post applications.

TABLE 7—PB, PBES AND PBS POST CAP ALLOWABLE LOADS1,2,3,4,5

			DIME	NOION	. (! \			FAS1	ENER S	SCHEDULE	ALLOWABLE LOADS (lbs)			
STOCK NO.	STEEL		DIME	NSIONS	5 (In.)			Qty			Uplift	F1	F2	
STOCK NO.	GAGE	W1	Н1	H2	L1	L2	Po	st	Beam	Type	C _D = 1.6	C _D = 1.6	C _D = 1.6	
		VVI	п	112	LI	LZ	P_{W}	PL	BL		G _D = 1.6	C _D = 1.0	O _D = 1.0	
PB44-6	18	11/2	21/8	1 ¹ / ₂	35/8	35/8	8	0	8	16d Common	585	1,760	1,015	
PB66-6	18	11/2	21/2	3	5 ⁹ / ₁₆	5 ⁹ / ₁₆	8	0	8	16d Common	585	1,760	1,015	
PBS44	18	1 ⁷ / ₁₆	2 ⁵ / ₁₆	2 ¹³ / ₁₆	39/16	6 ¹ / ₂	8	4	12	16d Common	2,650	1,860	1,110	
PBS44R	18	1 ¹ / ₂	2 ⁵ / ₁₆	2 ³ / ₁₆	4	7	4	4	8	16d Common	1,765	920	810	
PBS66	18	11/4	2 ⁵ / ₁₆	27/8	5 ¹ / ₂	8	8	6	12	16d Common	2,015	1,865	1,300	
PBS66R	18	11/4	2 ⁵ / ₁₆	2 ³ / ₁₆	6	8 ¹ / ₂	4	6	10	16d Common	1,670	1,190	1,235	
PBES44	18	11/2	2 ³ / ₈	23/4	31/4	43/4	4	4	8	16d Common	1,765	920	810	
PBES66	18	11/2	2 ³ / ₈	21/8	5 ¹ / ₂	7	4	4	8	16d Common	1,670	1,190	1,235	

⁵ PB, PBES, and PBS post caps must be installed in pairs, as illustrated in the following figure. Required fastener schedules and tabulated allowable loads are per pair of connectors.



¹Allowable loads have been adjusted for a load factor, C_D, of 1.6, corresponding to a ten-minute load duration (i.e., wind or earthquake loading), in accordance with the NDS. The allowable loads do not apply to loads of other durations. See Section 4.1 and 4.2 for additional design and installation requirements.

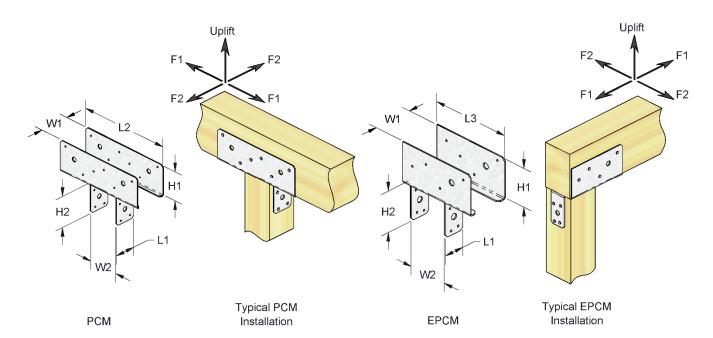
²See section 3.10.3 for required fastener dimensions and mechanical properties.

³Allowable loads shown are for installations in wood members complying with Section 3.10.2.
⁴ "F1" loads are parallel to the major axis of the beam, and "F2" loads are perpendicular to the major axis of the beam.

TABLE 8—PCM AND EPCM POST CAP ALLOWABLE LOADS 1,2,3,4

STOCK	STEEL			DIME	NSION	S (in.)			FA	STEN	ER SCH	EDULE	PCM AL	LOWABLE (lbs.)	LOADS	EPCM AI	LLOWABLE (lbs.)	LOADS
NO.	GAGE	W ₁	14/						Qty	Qty	Beam	T	Uplift	F1	F2	Uplift	F1	F2
		VV1	W ₂	L ₁	L ₂	L ₃	H₁	H ₂	Post	РСМ	EPCM	Type	C _D = 1.6					
PCM4416	16	39/16	39/16	27/16	11	71/4	39/16	33/4	8	12	8	16d Com.	970	1,115	1,335	970	1,115	1,335
PCM4516	16	39/16	5 ⁵ / ₁₆	27/16	12 ³ / ₄	9	39/16	33/4	8	12	8	16d Com.	970	1,115	1,335	970	1,115	1,335
PCM4616	16	39/16	5 ⁹ / ₁₆	27/16	13	91/4	39/16	33/4	8	12	8	16d Com.	970	1,115	1,335	970	1,115	1,335
PCM4716	16	39/16	71/8	2 ⁷ / ₁₆	14 ⁹ / ₁₆	10 ¹³ / ₁₆	39/16	33/4	8	12	8	16d Com.	970	1,115	1,335	970	1,115	1,335
PCM4816	16	39/16	7 ⁹ / ₁₆	27/16	15	11 ¹ / ₄	39/16	33/4	8	12	8	16d Com.	970	1,115	1,335	970	1,115	1,335
PCM5416	16	5 ⁹ / ₁₆	39/16	313/16	11	71/4	35/8	33/4	8	12	8	16d Com.	970	1,115	1,335	970	1,115	1,335
PCM5516	16	5 ⁹ / ₁₆	5 ⁹ / ₁₆	313/16	12 ³ / ₄	9	35/8	33/4	8	12	8	16d Com.	970	1,115	1,335	970	1,115	1,335
PCM6416	16	5 ¹ / ₂	39/16	313/16	11	71/4	31/2	33/4	8	12	8	16d Com.	950	1,545	1,675	950	1,545	1,675
PCM6616	16	5 ¹ / ₂	5 ⁹ / ₁₆	3 ¹³ / ₁₆	13	91/4	31/2	33/4	8	12	8	16d Com.	950	1,545	1,675	950	1,545	1,675
PCM6816	16	5 ¹ / ₂	79/16	313/16	15	111/4	31/2	33/4	8	12	8	16d Com.	950	1,545	1,675	950	1,545	1,675
PCM8416	16	71/2	39/16	5 ⁵ / ₈	11	71/4	31/2	33/4	8	12	8	16d Com.	950	1,545	1,675	950	1,545	1,675
PCM8616	16	71/2	5 ⁹ / ₁₆	5 ⁵ / ₈	13	91/4	33/8	33/4	8	12	8	16d Com.	950	1,545	1,675	950	1,545	1,675
PCM8816	16	71/2	79/16	5 ⁵ / ₈	15	11 ¹ / ₄	31/2	$3^{3}/_{4}$	8	12	8	16d Com.	950	1,545	1,675	950	1,545	1,675
PCM44	12	39/16	39/16	27/16	11	71/4	39/16	33/4	8	12	8	16d Com.	1,665	1,350	1,890	1,665	1,350	1,890
PCM46	12	39/16	5 ⁹ / ₁₆	27/16	13	91/4	39/16	33/4	8	12	8	16d Com.	1,665	1,350	1,890	1,665	1,350	1,890
PCM48	12	39/16	79/16	27/16	15	11 ¹ / ₄	39/16	33/4	8	12	8	16d Com.	1,665	1,350	1,890	1,665	1,350	1,890
PCM64	12	5 ¹ / ₂	39/16	3 ¹³ / ₁₆	11	71/4	31/2	33/4	8	12	8	16d Com.	1,500	1,875	1,915	1,500	1,875	1,915
PCM66	12	5 ¹ / ₂	5 ⁹ / ₁₆	3 ¹³ / ₁₆	13	91/4	31/2	33/4	8	12	8	16d Com.	1,500	1,875	1,915	1,500	1,875	1,915
PCM68	12	5 ¹ / ₂	79/16	313/16	15	11 ¹ / ₄	31/2	33/4	8	12	8	16d Com.	1,500	1,875	1,915	1,500	1,875	1,915
PCM75	12	71/8	5 ⁵ / ₁₆	5 ⁵ / ₈	12 ³ / ₄	10	311/16	33/4	8	12	8	16d Com.	1,500	1,875	1,915	1,500	1,875	1,915
PCM77	12	71/8	71/8	5 ⁵ / ₈	14 ⁹ / ₁₆	10 ¹³ / ₁₆	311/16	33/4	8	12	8	16d Com.	1,500	1,875	1,915	1,500	1,875	1,915
PCM84	12	71/2	39/16	5 ⁵ / ₈	11	71/4	31/2	33/4	8	12	8	16d Com.	1,500	1,875	1,915	1,500	1,875	1,915
PCM86	12	71/2	5 ⁹ / ₁₆	5 ⁵ / ₈	13	91/4	31/2	33/4	8	12	8	16d Com.	1,500	1,875	1,915	1,500	1,875	1,915
PCM88	12	71/2	79/16	5 ⁵ / ₈	15	11 ¹ / ₄	31/2	33/4	8	12	8	16d Com.	1,500	1,875	1,915	1,500	1,875	1,915

^{4&}quot;F1" loads are parallel to the major axis of the beam, and "F2" loads are perpendicular to the major axis of the beam.



 $^{^{1}}$ Allowable loads have been adjusted for a load factor, C_D , of 1.6, corresponding to a ten-minute load duration (i.e., wind or earthquake loading), in accordance with the NDS. The allowable loads do not apply to loads of other durations. See Sections 4.1 and 4.2 for additional design and installations requirements.

²See Section 3.10.3 for required fastener dimensions and mechanical properties.

³Allowable loads shown are for installations in wood members complying with Section 3.10.2.

TABLE 9—PBC POST BEAM CORNER CONNECTOR ALLOWABLE LOADS^{1,2,3,4}

STOCK	STEEL	DIMENS (in.)			FASTENE	ER SCHEDULE	ALL	OWABLE LO	ADS
NO.	GA.			(Qty	_ 2	Uplift	F1	F2
		W	L	Post	Beam	Type ²	160%	160%	160%
PBC44	18	4 ¹⁵ / ₁₆	6 ¹ / ₂	8	8	16d Common	1,765	1,520	1,520
PBC66	18	6 ¹⁵ / ₁₆	6 ¹ / ₂	8 8		16d Common	1,765	1,520	1,520

³Allowable loads shown are for installations in wood members complying with Section 3.10.2.

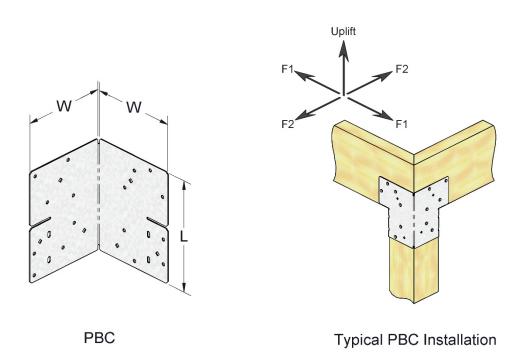


FIGURE 9—PBC POST BEAM CONRNER CONNECTORS

¹Allowable loads have been adjusted for a load factor, C_D, of 1.6, corresponding to a ten-minute load duration (i.e., wind or earthquake loading), in accordance with the NDS. The allowable loads do not apply to loads of other durations. See Sections 4.1 and 4.2 for additional design and installations requirements.

²See Section 3.10.3 for required fastener dimensions and mechanical properties.

TABLE 10—STEEL TYPE, GRADE AND CORROSION RESISTANCE

PRODUCT	STEEL	COATING
C Post Cap	No. 18 Ga. Material: ASTM A653, SS designation, Grade 40	G90 ¹ , G185 ¹
D Post Anchor	No. 18 Ga. Material: ASTM A653, SS designation, Grade 40	G90 ¹ , G185 ¹
KCC and KECC Column Caps	No. 7 Ga. Material: ASTM A1011, SS designation, Grade 40 No. 3 Ga. Material: ASTM A36	Painted
KCCQ / KECCQ Column Cap	No. 3 Ga. Material: ASTM A36 No. 7 Ga. Material: ASTM A 1011, SS designation, Grade 40	Painted
PA Post Anchor	No. 18 Ga. Material: ASTM A653, SS designation, Grade 40	G90 ¹ , G185 ¹
PAU Post Anchor	Nos. 10, 12, 16 Ga. Material: ASTM A653, SS designation, Grade 40 Nos. 3, 10 Ga. Material: ASTM A36	G90¹, G185¹ Painted
PB, PBES and PBS Post Caps	No. 18 Ga. Material: ASTM A653, SS designation, Grade 40	G90 ¹ , G185 ¹
PCM Post Cap	Nos. 12 and 16 Ga. Material: ASTM A653, SS designation, Grade 40	G90 ¹ , G185 ¹
PBC Post Beam Corner Connector	No. 18 Ga. Material: ASTM A653, SS designation, Grade 40	G90 ¹ , G185 ¹

¹Corrosion protection is a zinc coating in accordance with ASTM A653.

TABLE 11—CROSS-REFERENCE OF PRODUCT NAMES WITH APPLICABLE REPORT SECTIONS, TABLES AND FIGURES

PRODUCT NAME	REPORT SECTION	TABLE NO.	FIGURE NO.	
C Post Cap	3.1	1	1	
D Post Anchor	3.2	2	2	
EPCM Post Cap	3.8	8	8	
KCC Column Cap ¹	3.3	3	3	
KECC Column Cap ¹	3.3	3	3	
KCCQ Column Cap ¹	3.4	4	4	
KECCQ Column Cap 1	3.4	4	4	
PA Post Anchor	3.5	5	5	
PAU Post Anchor	3.6	6	6	
PB, PBES and PBS Post Caps	3.7	7	7	
PCM Post Cap	3.8	8	8	
PBC Post Beam Corner Connector	3.9	9	9	

¹Products are manufactured at the MiTek manufacturing facilities in Largo, FL; Tolleson, AZ; and Montgomery, MN under a quality control program with third-party inspections by ICC-ES.

TABLE 12—ADDITIONAL LISTEE MODEL NUMBER CROSS-REFERENCE FOR EVERBILT (A BRAND OF THE HOME DEPOT) WITH APPLICABLE REPORT SECTIONS, TABLES AND FIGURES

MITEK MODEL NUMBER	EVERBILT MODEL NUMBER	REPORT SECTION	TABLE NO.	FIGURE NO.
C44-TZ	UPB4Z3	3.1	1	1
C66-TZ	HPB6Z3	3.1		
D44-TZ	HPB4Z3	3.2	2	2
D66-TZ	PC6Z3	3.2		
PA66E-TZ	APB66Z3	2.5	5	5
PA44E-TZ	APB44Z3	3.5		



ICC-ES Evaluation Report

ESR-3449 LABC and LARC Supplement

Reissued October 2022

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A Subsidiary of the International Code Council®

DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23—Wood, Plastic, and Composite Fastenings

REPORT HOLDER:

MITEK® INC.

EVALUATION SUBJECT:

MITek® STRUCTURAL CONNECTORS: CAPS AND BASES

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that MiTek® structural connectors: caps and bases, described in ICC-ES evaluation report <u>ESR-3449</u>, have also been evaluated for compliance with the codes noted below as adopted by the Los Angeles Department of Building and Safety (LADBS).

Applicable code editions:

- 2020 City of Los Angeles Building Code (LABC)
- 2020 City of Los Angeles Residential Code (LARC)

2.0 CONCLUSIONS

The MiTek® structural connectors: caps and bases, described in Sections 2.0 through 7.0 of the evaluation report <u>ESR-3449</u>, comply with the LABC Chapter 23, and the LARC, and are subjected to the conditions of use described in this supplement.

3.0 CONDITIONS OF USE

The MiTek® structural connectors: caps and bases, described in this evaluation report supplement must comply with all of the following conditions:

- All applicable sections in the evaluation report <u>ESR-3449</u>.
- The design, installation, conditions of use and labeling are in accordance with the 2018 *International Building Code*® (2018 IBC) provisions noted in the evaluation report <u>ESR-3449</u>.
- The design, installation and inspection are in accordance with additional requirements of LABC Chapters 16 and 17, as applicable.
- The connections are not approved to resist uplift forces from wood shear walls.
- Allowable loads must be reduced when load durations with lower value Cd (than what is in the tables) govern.
- Under the LARC, an engineered design in accordance with LARC Section R301.1.3 must be submitted.
- The hillside building provisions in LABC Section 2301.1 are excluded from this supplement.

This supplement expires concurrently with the evaluation report, reissued October 2022.





ICC-ES Evaluation Report

ESR-3449 FBC Supplement

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES Section: 06 05 23—Wood, Plastic, and Composite Fastenings

REPORT HOLDER:

MITEK® INC.

EVALUATION SUBJECT:

MITek® STRUCTURAL CONNECTORS: CAPS AND BASES

1.0 REPORT PURPOSE AND SCOPE

Purpose:

The purpose of this evaluation report supplement is to indicate that MiTek caps and bases, for connecting wood framing members, described in ICC-ES evaluation report ESR-3449, have also been evaluated for compliance with the codes noted below.

Applicable code editions:

- 2020 Florida Building Code—Building
- 2020 Florida Building Code—Residential

2.0 CONCLUSIONS

The MiTek caps and bases, described in Sections 2.0 through 7.0 of the evaluation report ESR-3449, comply with the *Florida Building Code—Building Code—Building Code—Residential*, provided the design requirements are determined in accordance with the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable. The installation requirements noted in ICC-ES evaluation report ESR-3449 for the 2018 *International Building Code®* meet the requirements of the *Florida Building Code—Building* or the *Florida Building Code—Residential*, as applicable.

Use of the MiTek caps and bases has also been found to be in compliance with the High-Velocity Hurricane Zone (HVHZ) provisions of the *Florida Building Code—Building*, and the *Florida Building Code—Residential* with the following condition:

a. For connections subject to uplift, the connection must be designed for no less than 700 pounds (3114 N).

For products falling under Florida Rule 61G20-3, verification that the report holder's quality-assurance program is audited by a quality-assurance entity approved by the Florida Building Commission for the type of inspections being conducted is the responsibility of an approved validation entity (or the code official, when the report holder does not possess an approval by the Commission).

This supplement expires concurrently with the evaluation report ESR-3449, reissued October 2022.

